

# Symmetry breaking, snap-through and pull-in instabilities under dynamic loading of microelectromechanical shallow arches

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## Abstract

Arch-shaped microelectromechanical systems (MEMS) have been used as mechanical memories, micro-relays, micro-valves, optical switches and digital micro-mirrors. A bi-stable structure, such as an arch, is characterized by a multivalued load deflection curve. Here we study the symmetry breaking, the snap-through instability and the pull-in instability of a bi-stable arch-shaped MEMS under static and dynamic electric loads. Unlike a mechanical load, the electric load is a nonlinear function of the *a priori* unknown deformed shape of the arch. The nonlinear partial differential equation governing transient deformations of the arch is solved numerically using the Galerkin method and a time integration scheme that adaptively adjusts the time step to compute the solution within the prescribed tolerance. For the static problem, the displacement control and the pseudo-arc-length continuation methods are used to obtain the bifurcation curve of the arch's displacement versus a load parameter. The displacement control method fails to compute the arch's asymmetric deformations that are found by the pseudo-arc-length continuation method. For the dynamic problem, two distinct mechanisms of the snap-through instability are found. It is shown that critical loads and geometric parameters for instabilities of an arch under an electric load with and without consideration of mechanical inertia effects are quite different. A phase diagram between a critical load parameter and the arch height is constructed to delineate different regions of instabilities. We compare results from the present model with those from a continuum mechanics based approach, and with results of other models and experiments available in the literature.

(Some figures in this article are in colour only in the electronic version)

## 1. Introduction

An electrically actuated microelectromechanical system (MEMS) consists of a deformable electrode made of a conductive material suspended above a rigid conductive electrode with a dielectric medium, generally air, between them (figure 1). An electric potential difference applied between the two electrodes induces the Coulomb pressure on the electrodes, which deflects the deformable electrode towards the rigid one. The elastic restoring force induced in the deformed electrode restricts its motion. Electric charges redistribute on the deformable electrode's surface as the gap between it and the rigid electrode decreases, which in turn increases the Coulomb pressure and

deflects the deformable electrode more until the Coulomb pressure balances the elastic restoring force. MEMS of dimensions in the range of a few to a hundred micrometers are used as radio-frequency (RF) switches, varactors and inductors [37], accelerometers [38], pressure sensors, controllers for micro-mirrors [42], micro-pumps [4] and bio-MEMS [2].

### 1.1. The pull-in instability in MEMS

For electrically actuated MEMS, the applied electric potential has an upper limit, beyond which the elastic restoring force does not balance the corresponding Coulomb force, resulting in collapse of the deformable electrode onto the rigid one.

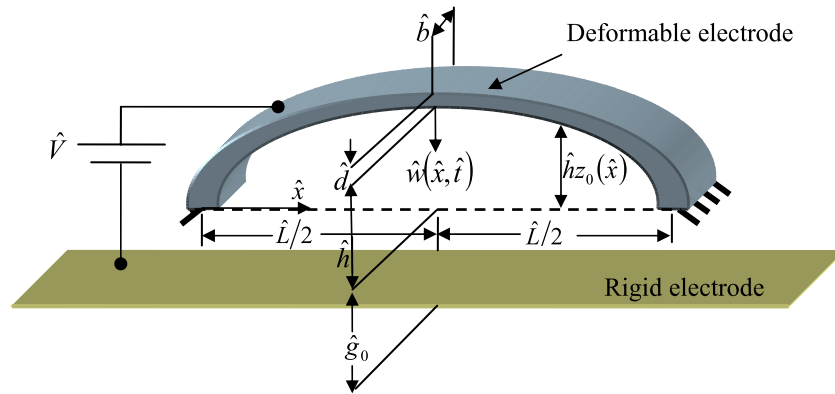


Figure 1. Schematic sketch of the problem studied.

This phenomenon, called the pull-in instability, was observed experimentally by Taylor [41] and Nathanson *et al* [28]. The corresponding values of the potential difference and the peak deflection of the deformable electrode are called the pull-in voltage and the pull-in deflection, respectively. Collectively, they are known as the pull-in parameters.

Accurate estimates of pull-in parameters are crucial for designing electrically actuated MEMS. In switching applications [30], the pull-in instability is necessary for the switch to operate. However, for micro-mirrors [20] and micro-resonators [26] the pull-in instability restricts the range of operational displacement of the device.

The literature on MEMS is reviewed in [47].

### 1.2. The snap-through instability in an arch-shaped MEMS

In an arch-shaped deformable electrode such as that shown in figure 1, in addition to the pull-in instability, the snap-through instability can occur under the Coulomb pressure. Depending upon the initial elevation  $\hat{h}$ , either snap-through or pull-in instability or both instabilities occur. Advantages of the snap-through instability have been exploited in actuators [43, 44, 39, 31], micro-valves [17] and transducers [25]. The snap-through instability of an arch-shaped MEMS under slowly applied electric loads has been observed experimentally and studied with reduced order models in [46, 23, 24, 21]. Various conditions such as arch rise  $\hat{h}$  (see figure 1), arch thickness  $\hat{d}$ , type of loads (step or ramp) and gap  $\hat{g}_0$  between the electrodes determine whether the snap-through will occur. Pippard [34] and Patricio *et al* [32] have presented a phase diagram between the arch length and the initial arch angle at the clamped end, showing conditions for which the snap-through can occur due to a quasistatic deflection-independent point load. Krylov *et al* [21] have presented a phase diagram between  $\hat{h}$  and  $\hat{d}$  depicting conditions for which the snap-through can occur in static deflections of a bell-shaped MEMS. Depending on the arch shape and the load type, the following three scenarios arise: either only the pull-in instability occurs, or the arch undergoes the snap-through and then the pull-in instability, or the snap-through and the pull-in happen simultaneously. In each case, the pull-in instability occurs.

### 1.3. The dynamic pull-in and snap-through instabilities

The pull-in instability in an MEMS under a transient electric load has been analyzed in [29, 35, 14, 11, 3, 22, 10], and the snap-through of arches and shells during their transient deformations under deformation-independent mechanical loads has been analyzed in [19, 9, 27, 40, 16, 18]. The ‘dynamic snap-through’ generally means a large increase in response resulting from a small increase in a load parameter [19]. Conditions for the dynamic snap-through to occur depend on the geometric parameters of the arch and on the load types. Here we find the arch height  $\hat{h}$  and the load parameter  $\beta$  for which the snap-through instability will occur under a step electric potential difference. We also study different mechanisms of the snap-through instability.

## 2. Mathematical model

The governing equation for a shallow micro-arch under an electrostatic load in terms of non-dimensional variables is [21]

$$\ddot{w} + c\dot{w} + w^{IV} - \alpha(hz_0'' - w'') \int_0^1 (2hz_0'w' - (w')^2) dx = \frac{\beta}{(1 + hz_0 - w)^2}, \quad x \in (0, 1). \quad (1)$$

The boundary and initial conditions for a fixed-fixed arch initially at rest are

$$w(0, t) = w(1, t) = w'(0, t) = w'(1, t) = w(x, 0) = \dot{w}(x, 0) = 0. \quad (2)$$

In equations (1) and (2), a superimposed dot and a prime denote a derivative with respect to time  $t$  and the space coordinate  $x$ , respectively,  $\hat{\epsilon}_0 = 8.854 \times 10^{-12}$  F m<sup>-1</sup> the vacuum permittivity,  $\hat{\rho}$  the mass density,  $x = \hat{x}/\hat{L}$ ,  $w = \hat{w}/\hat{g}_0$ , the transverse displacement,  $d = \hat{d}/\hat{g}_0$ ,  $b = \hat{b}/\hat{g}_0$ ,  $h = \hat{h}/\hat{g}_0$ ,  $\alpha = \hat{g}_0^2 \hat{b} \hat{d} / 2 \hat{I}$  the stretching ratio,  $c = \hat{c} \hat{L}^2 / \sqrt{\hat{\rho} \hat{b} \hat{d} \hat{E} \hat{I}}$ ,  $\hat{c}$  the damping coefficient,  $t = \hat{t} \sqrt{(\hat{E} \hat{I}) / (\hat{\rho} \hat{b} \hat{d} \hat{L}^4)}$ ,  $\hat{t}$  the dimensional time,  $\beta = \hat{\epsilon}_0 \hat{b} \hat{L}^4 \hat{V}^2 / 2 \hat{E} \hat{I} \hat{g}_0^3$  the potential difference parameter,  $\hat{\epsilon}_0$  the vacuum permittivity,  $\hat{b}$  the arch width,  $\hat{L}$  the arch length,  $\hat{g}_0$  the initial gap,  $\hat{V}$  the electric potential difference between

the two electrodes,  $\hat{E}$  Young's modulus of the arch material,  $\hat{I} = \hat{b}\hat{d}^3/12$  the second moment of the cross section of the arch about its centroidal axis and  $z_0(x)$  the initial shape of the arch. As discussed in [10] the damping provided by deformations of the air between the two electrodes can be approximated by the term  $c\dot{w}$ .

We solve equation (1) using the Galerkin method by approximating the transverse displacement  $w$  by the series

$$w(x, t) \approx \sum_{i=1}^n q_i(t) \phi_i(x) \quad (3)$$

where  $q_i(t)$  are the generalized coordinates and  $\phi_i(x)$  eigenmodes of an undamped fixed-fixed straight beam:

$$\phi_i(x) = J_i \left( \frac{\cos(\lambda_i x) - \cosh(\lambda_i x)}{-\sin(\lambda_i) - \sinh(\lambda_i)} - \frac{\sin(\lambda_i x) - \sinh(\lambda_i x)}{\cos(\lambda_i) - \cosh(\lambda_i)} \right) \quad (4)$$

where  $\lambda_i$  is a solution of

$$\cos(\lambda_i) \cosh(\lambda_i) - 1 = 0. \quad (5)$$

The normalization coefficient  $J_i$  is such that  $\text{Max}(\phi_i(x)) = 1$  for all  $x \in (0 \leq x \leq 1)$  and  $n$  equals the number of modes used in the approximation (3).

Substituting equation (3) into equation (1), multiplying both sides of the resulting equation with  $\phi_i(x)$ , integrating it over the domain (0, 1), integrating by parts terms involving  $w^{IV}$ ,  $z_0''$  and  $w''$ , and using  $\phi(0) = \phi(1) = \phi'(0) = \phi'(1) = 0$  we get (see e.g., [48])

$$\mathbf{M}\ddot{\mathbf{q}} + \mathbf{C}\dot{\mathbf{q}} + \mathbf{B}\mathbf{q} + 2\alpha h^2 \mathbf{z}_0^T \mathbf{q} \mathbf{z}_0 - \alpha h \mathbf{q}^T \mathbf{S} \mathbf{q} \mathbf{z}_0 - 2\alpha h \mathbf{z}_0^T \mathbf{q} \mathbf{S} \mathbf{q} + \alpha \mathbf{q}^T \mathbf{S} \mathbf{q} \mathbf{S} \mathbf{q} = \beta \mathbf{F} \quad (6)$$

where

$$M_{ij} = \int_0^1 \phi_i \phi_j \, dx \quad (7)$$

$$C_{ij} = c M_{ij} \quad (8)$$

$$B_{ij} = \int_0^1 \phi_i'' \phi_j'' \, dx \quad (9)$$

$$z_{0i} = \int_0^1 z_0' \phi_i' \, dx \quad (10)$$

$$S_{ij} = \int_0^1 \phi_i' \phi_j' \, dx \quad (11)$$

$$F_i = \int_0^1 \frac{\phi_i}{(1 + h z_0(x) - \sum_{i=1}^n q_i(t) \phi_i(x))^2} \, dx. \quad (12)$$

Equation (6) is nonlinear in  $\mathbf{q}$  because the fifth, sixth and seventh terms on its left-hand side are nonlinear in  $\mathbf{q}$  and the load vector on its right-hand side is a nonlinear function of  $\mathbf{q}$ . Boundary conditions in equation (2) are identically satisfied by the assumed form (3) for  $w(x, t)$ . Initial conditions in equation (3) require that  $\mathbf{q}(0) = \mathbf{0}$  and  $\dot{\mathbf{q}} = \mathbf{0}$ . Non-dimensional variables affecting the arch deformation are  $\alpha$ ,  $c$ ,  $h$ ,  $\beta$  and the function  $z_0'(x)$ .

In subsequent sections, unless stated otherwise, the arch shape is described by  $z_0(x) = \sin^2(\pi x)$ . Thus, the initial slope of the arch at each clamped end is zero.

### 3. Results and discussions

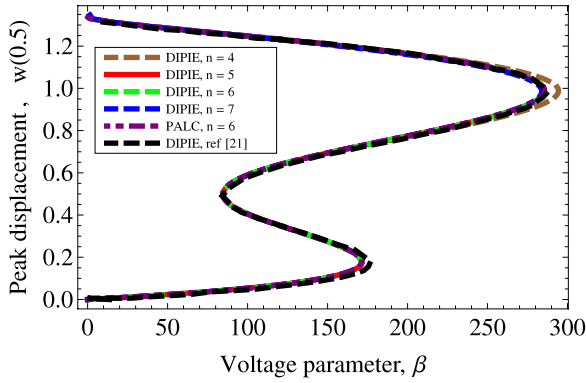
We have developed a computer code to solve equation (6) numerically by writing it in the state space form, and using the software Livermore Solver for ordinary differential equations (LSODE) [36]. The relative and the absolute tolerances in LSODE are set equal to  $10^{-6}$  and the parameter MF = 22 to use the numerically computed Jacobian matrix. When solving a static problem we neglect the time dependence of  $\mathbf{q}$  and the first and the second terms on the left-hand side of equation (6), and then solve it using a displacement control approach (displacement iteration pull-in extraction (DIPIE) algorithm [8]) and the pseudo-arc-length continuation (PALC) algorithm [13]. We implement the DIPIE algorithm using the nonlinear equation solver FINDROOT in the commercial computer code Mathematica, and use the freely available software AUTO [1] for the PALC algorithm.

#### 3.1. Validation and convergence study

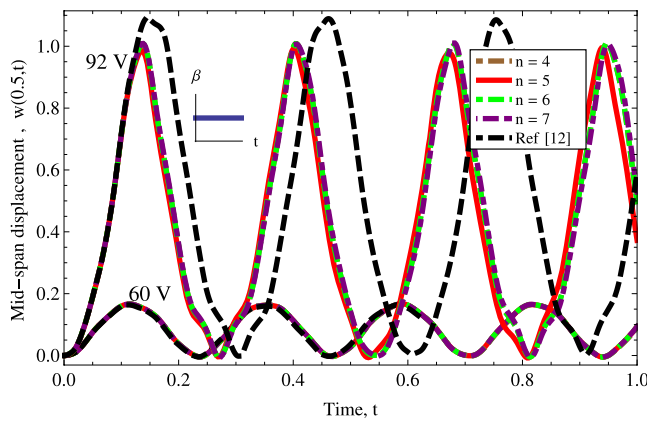
The mathematical model described above has been validated by comparing computed results for four sample problems with those reported in the literature and obtained from a continuum-mechanics-based approach. First, we consider static deformations of an arch with  $h = 0.3$  and  $\alpha = 121.5$ . Figure 2 exhibits the bifurcation curve for the arch found with the DIPIE algorithm, the PALC algorithm and the bifurcation curve for a static problem reported in [21]. Furthermore, results with the PALC and the DIPIE algorithms coincide with each other, and for the DIPIE algorithm the bifurcation curves computed by taking  $n = 5, 6$  and 7 in equation (3) are nearly indistinguishable from each other. These results agree very well with those in [21] obtained with the DIPIE algorithm. We also investigated the effect of the number  $n$  of terms in equation (3) upon critical values of  $\beta$  and of the peak displacements at the snap-through and the pull-in instabilities. Critical values of  $\beta$  and of the peak displacements were found to converge as  $n$  increases. For  $n \geq 6$ , the change in the critical values of  $\beta$  and the peak displacements at the two instabilities is less than 1%. Unless noted otherwise, results presented below are for  $n = 6$  in equation (3). Arches studied herein have only one stable shape, namely the initial shape, when the power is lost ( $\beta = 0$ ). However, for a range of values of  $\beta$  (e.g.  $70 < \beta < 170$ , see figure 2) the arch has two stable equilibrium positions for the same value of  $\beta$ .

Referring to figure 1, we consider a bell-shaped silicon arch with  $\hat{L} = 1$  mm,  $\hat{b} = 30$   $\mu\text{m}$ ,  $\hat{d} = 2.4$   $\mu\text{m}$ ,  $\hat{g}_0 = 10.1$   $\mu\text{m}$ ,  $\hat{h} = 3.0$   $\mu\text{m}$  and its bottom surface described by  $z_0 = \sin^2(\pi \hat{x}/\hat{L})$ . Values assigned to material parameters are  $\hat{E} = 169$  GPa and Poisson's ratio  $\nu = 0.3$ . We analyze it as a plane strain problem by using the effective Young's modulus  $\hat{E} = \hat{E}/(1 - \nu^2)$ , and account for fringing fields by computing the Coulomb pressure  $F_i$  (cf equation (12)) according to the Mejis-Fokkema formula [6, 21]:

$$F_i = \int_0^1 \frac{\phi_i}{(1 + h z_0(x) - w(x))^2} F_g \, dx \quad (13)$$



**Figure 2.** Bifurcation curve of the peak deflection of the mid-span of the arch versus the applied potential difference parameter for the static problem.

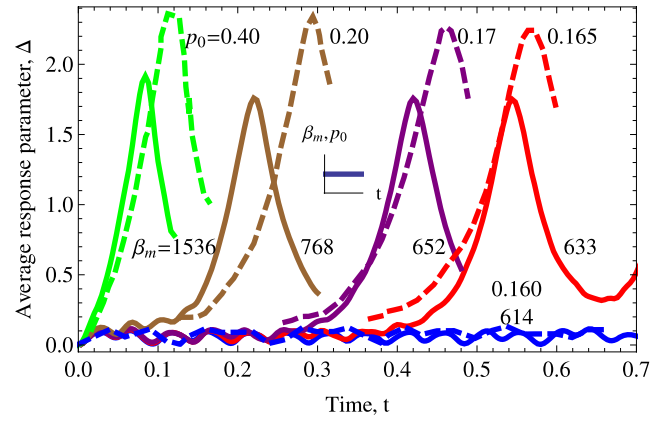


**Figure 3.** Time histories of the peak deflection of the mid-span of the arch due to 60 V ( $\beta = 72$ ) and 92 V ( $\beta = 170$ ) step potential difference between the rigid electrode and the arch.

where

$$F_g = \left( 1 + 0.265 \left( \frac{1 + z_0(x) - w(x)}{b} \right)^{3/4} + 0.53 \left( \frac{d}{b} \sqrt{\frac{1 + z_0(x) - w(x)}{d}} \right) \right). \quad (14)$$

For different numbers of terms in equation (3), figure 3 shows the peak displacement  $w(0.5, t)$  versus time  $t$  for 60 and 92 V step potential differences and those reported in [12]. Time histories of the peak displacement for 60 V step electric potential difference agree well with those reported in [12]. However, for 92 V potential difference, the maximum peak displacement and the time period of oscillation are 8% and 20% lower, respectively, than those reported in [12]. In [12], the problem is analyzed using the coupled finite element and boundary element methods assuming that the arch is in a plane strain state of deformation, the electric load is computed after every time step accounting for deformations of the arch and the electric load is applied normal to the deformed surface of the arch. Here the Coulomb pressure is found by using the parallel plate approximation (PPA) [7] and it acts vertically downwards. In section 3.5, a detailed comparison of results



**Figure 4.** Time histories of the average response parameter for the arch ( $h = 1.0$ ) for different values of  $\beta_m = (\hat{d}^2 \hat{L}^2 \hat{b} / \hat{R}^2 \hat{I} \hat{h}) p_0$ . Although the value of  $\beta_m$  is different from  $p_0$  in the figure, they represent the same load. Solid curves are computed from the present model, dashed curves are from [19].

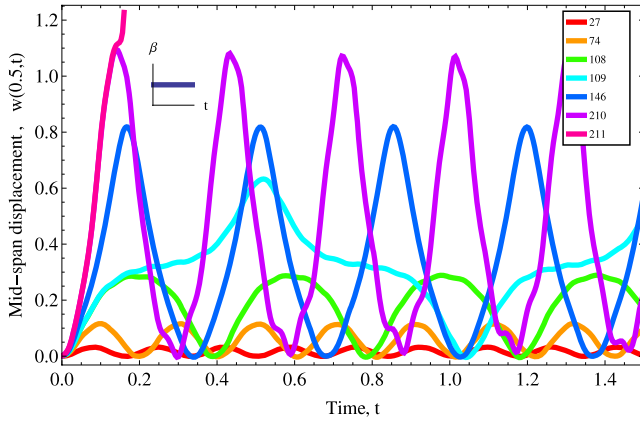
from the present method with those from the continuum-mechanics-based approach is given.

As results for the dynamic and the static problems found using  $n = 5, 6$  and  $7$  are virtually the same, henceforth we take  $n = 6$ .

We now compare results for a dynamic snap-through instability of a shallow arch with geometric imperfection (initial shape of the arch  $z_0(x) = (4 - 4x)x + 0.01d \sum_{i=2,4,6} \phi_i(x)$ ) under a mechanical pressure load with those reported in [19]. Equation (6) governs deformations of an arch under a mechanical pressure load if the potential difference parameter  $\beta$  is replaced by a load parameter  $\beta_m$  and the generalized force  $F_i$  (cf equation (12)) by  $F_i = \int_0^1 \phi_i dx$ . We use  $\hat{h}$  in place of  $\hat{g}_0$  to non-dimensionalize the deflection  $\hat{w}$  and the arch thickness  $\hat{d}$ . A different non-dimensional load parameter  $p_0 = \beta_m / (\hat{d}^2 \hat{L}^2 \hat{b} / \hat{R}^2 \hat{I} \hat{h})$  is used in [19].  $\hat{R}$  is the curvature of the arch.

Figure 4 exhibits time histories of the average response parameter  $\Delta(t) = \int_0^1 w(x, t) dx / \int_0^1 z_0(x) dx$  for the arch ( $h = 1.0, \alpha = 150$ ) for different values of  $\beta_m$ . A large change in the response of the arch is observed when  $\beta_m$  is increased from 614 ( $p_0 = 0.159$ ) to 615 ( $p_0 = 0.160$ ); the curve for  $\beta_m = 615$  is not depicted in figure 4. Therefore, the load  $\beta_m$  for the snap-through instability is between 614 and 615. In [19], this load is reported to be between  $p_0 = 0.160$  ( $\beta_m = 615$ ) and  $p_0 = 0.165$  ( $\beta_m = 633$ ). However, the maximum value of the average response parameter  $\Delta$  for  $p_0 = 0.165$  ( $\beta_m = 633$ ) from the present work is 21.5% lower than that reported in [19]. In [19], equation (6) is solved using an analog computer system with servo-multipliers providing the nonlinear terms. The solutions of frequency equation (5) and the mode shapes (cf equation (4)) are approximated such that values of different terms in equations (7)–(12) are accurate only to three significant digits. Here, we have used LSODE with the double precision arithmetic. We note that the presently computed response of the arch agrees qualitatively with that





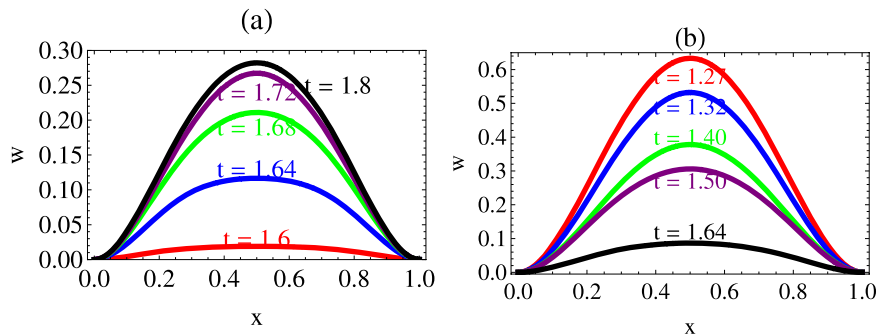
**Figure 5.** Time histories of the downward displacement of the mid-span of the fixed-fixed bell-shaped arch ( $h = 0.3$ ) for different values of  $\beta$ .

given in [19] and the two values of the load for the snap-through instability agree well with each other.

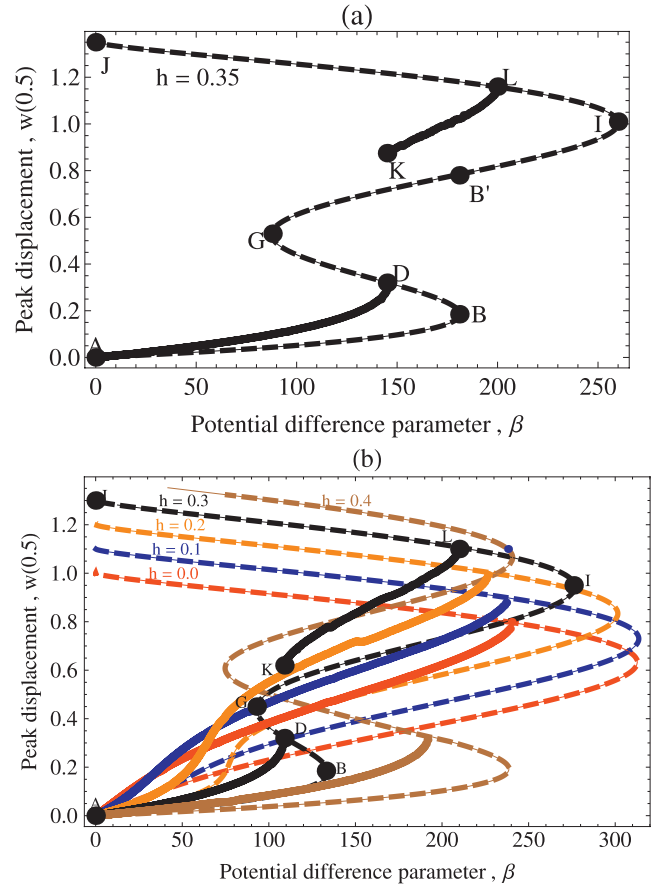
### 3.2. Direct snap-through instability

For a bell-shaped silicon arch with  $\alpha = 110$ ,  $h = 0.3$  and  $z_0(x) = \sin^2(\pi x)$ , figure 5 shows time histories of the mid-span deflection for different values of the applied step potential difference. A significant difference in the response of the arch occurs when  $\beta$  is increased from 108 to 109 in that the amplitude and the time period of oscillation increase noticeably. This sudden change in the response due to a small change in  $\beta$  indicates the snap-through instability. The critical value of  $\beta$  is between 108 and 109. By solving the problem for several values of  $\beta$  between 108 and 109 (e.g. obtained by the bisection method), one can compute a better value of  $\beta$  for the snap-through instability. The critical value of  $\beta$  for the pull-in instability is found to be between 210 and 211, since for  $\beta = 210$  the response of the arch remains bounded but for  $\beta = 211$  it becomes unbounded.

Figure 6 shows, at different times, snapshots of the deformed arch for  $\beta = 108$  (before the snap-through) and 109 (after the snap-through). It is clear that both before and after the snap-through the arch deforms symmetrically about the plane  $x = 0.5$ . Time histories of the generalized coordinates for  $\beta = 108$  and 109, not included here, show that



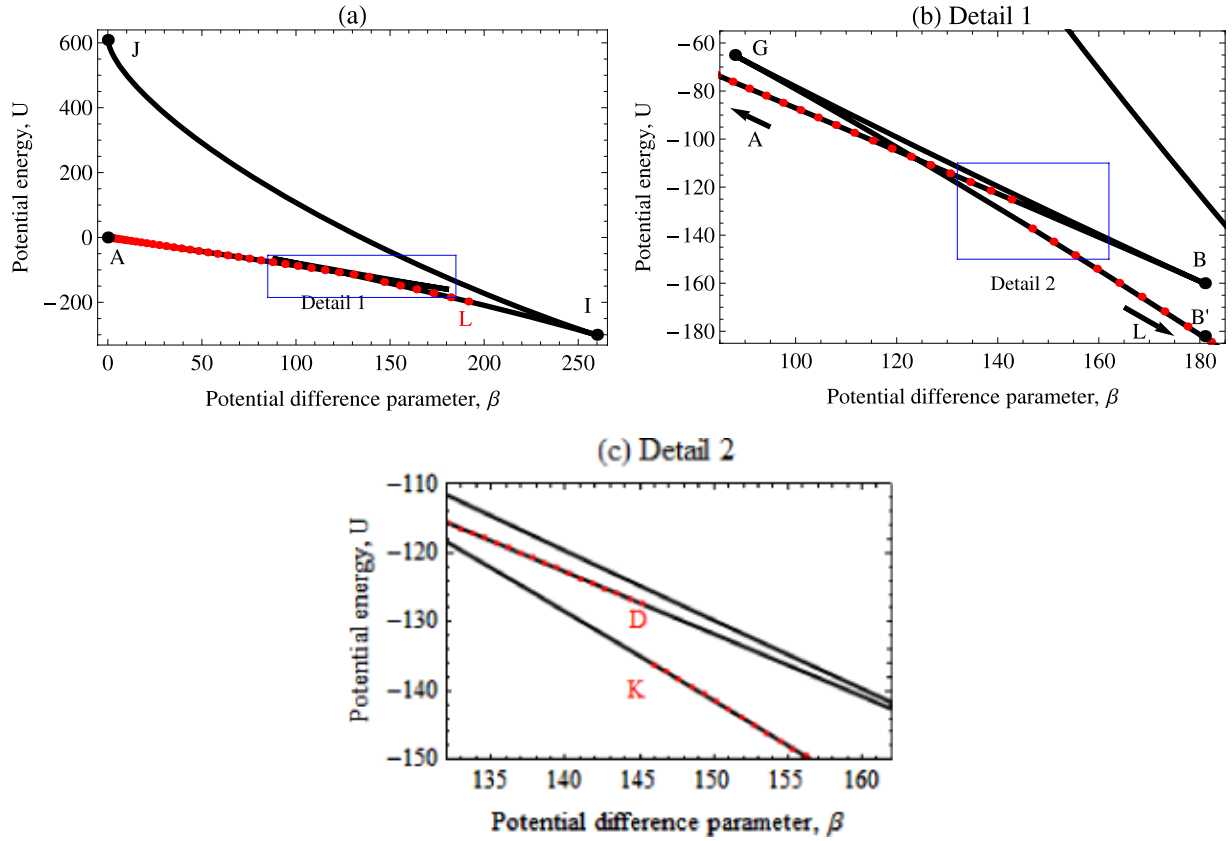
**Figure 6.** Snapshots of the displacement of the arch ( $h = 0.3$ ) for (a)  $\beta = 108$  and (b)  $\beta = 109$ .



**Figure 7.** Bifurcation diagrams ( $w(0.5)$  versus  $\beta$ ) of the arch for different arch heights for static problems. Dashed curves and light solid curves are results from the DIPIE and the PALC algorithms, respectively. Dark solid curves are the loci of the maximum displacement  $\text{Max}_{0 \leq t \leq 2} (w(0.5, t))$  under a step potential difference. Results from the DIPIE and the PALC algorithms overlap each other. (a) for  $h = 0.35$  clearly shows different curves.

participations of the asymmetric modes with coefficients  $q_2$ ,  $q_4$  and  $q_6$  are negligible as compared to those of the symmetric modes with coefficients  $q_1$ ,  $q_3$  and  $q_5$ . It is clear that a reasonably accurate response of the arch can be computed by considering only the fundamental mode in equation (3). A similar result for a beam is reported in [48].

For several values of the arch height, figure 7 depicts loci of the maximum deflection produced by different step



**Figure 8.** Potential energy  $U(\beta)$  versus the potential difference parameter  $\beta$  for  $h = 0.35$  for the static problem (black curve) and the locus of the minimum potential energy  $\text{Min}_{0 \leq t \leq 2}(U(\beta, t))$  as a function of the step potential difference for the dynamic problem (red dots).

electric potentials, and static bifurcation curves obtained by using the DIPIE and the PALC algorithms. In every case, the bifurcation curve obtained from the DIPIE algorithm coincides with that obtained from the PALC algorithm. We note that the snap-through due to a step electric potential difference occurs when the locus of the maximum deflection intersects the unstable branch of the static bifurcation curve (e.g. point D for  $h = 0.3$  and  $0.35$  in figures 7(a) and (b)). A similar observation is reported in [27] due to a displacement-independent mechanical pressure load wherein the snap-through is called the ‘direct snap-through’ because the external pressure directly induces snapping of the symmetric mode in contrast to the ‘indirect snapping’ induced due to the parametric excitation of asymmetric modes as reported in section 3.3.

We note that, after the snap-through instability (e.g. point D), the arch’s deformations are stable (e.g. point K) and the locus of the maximum displacement continues along the path KL until it again intersects the unstable branch of the static bifurcation curve (e.g. point L) when the pull-in instability occurs.

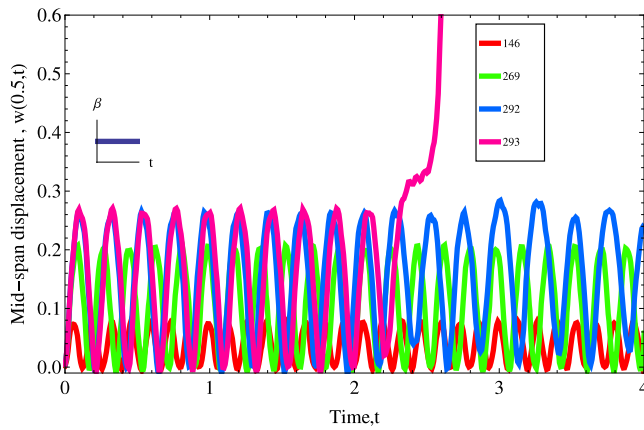
For  $h = 0.0$  and  $0.2$ , there is no snap-through instability, and the locus of the maximum deflection continues until the pull-in instability ensues. Therefore, a minimum height is required for the arch to undergo the snap-through instability; e.g. see section 3.4. For  $h = 0.4$  no stable configuration under a step electric potential difference is observed after the snap-through instability as the pull-in instability occurs immediately.

At the snap-through instability for  $h = 0.3$  and the step potential difference,  $\beta$  is  $\sim 15\%$  less and the peak displacement  $w$  is  $\sim 50\%$  more than their values for the corresponding static problem. At the pull-in instability,  $\beta$  is  $\sim 22\%$  less and the peak displacement  $\sim 21\%$  more than those for the static problem. Thus, the snap-through and the pull-in voltages for the dynamic problem are less than those for the static problem, but the snap-through and the pull-in deflections for the dynamic problem are greater than those for the static problem.

Figure 8 exhibits the variation of the potential energy versus  $\beta$  for a static problem with  $h = 0.35$  and  $\alpha = 110$ . The non-dimensional potential energy  $U$  is given by [21]

$$U = \frac{1}{2} \int_0^1 (w'')^2 dx + \frac{\alpha}{4} \left\{ \int_0^1 (2hz'_0 w' - (w')^2) dx \right\}^2 - \beta \int_0^1 \frac{dx}{(1 + hz_0 - w)}. \quad (15)$$

For a dynamic problem with  $\beta$  a step function of time,  $w$  and  $U$  are functions of time. In figure 8, we also plot as red dots the locus of the minimum values of  $U$  for given step values of  $\beta$ . Points A, B, D, etc. in figure 8 are for the same values of  $\beta$  as those in figure 7(a). For a static problem,  $U$  decreases from point A to point B as the value of  $\beta$  increases. At point B, the arch experiences the snap-through instability and the value of  $U$  suddenly drops from  $-160$  corresponding to point B to  $-182$  for point B' (see figure 8(b)). After the snap-through,  $U$  gradually decreases from  $-182$  at point B' to



**Figure 9.** For different applied step voltages time histories of the downward displacement of the mid-span of the fixed-fixed bell shaped arch with  $h = 0.5$ .

–301 at point I when the pull-in instability happens. For the dynamic problem, the locus of the minimum potential energy (red dots in figure 8) follows the curve of  $U$  versus  $\beta$  for the static problem from point A to point D. At point D, with a small increase in  $\beta$ , the minimum potential energy drops from –130 at point D to –138 at point K; this indicates the snap-through instability. For the static problem, the locus of the minimum potential energy follows the curve of  $U$  versus  $\beta$  from point K to point L where the pull-in instability happens (figure 8(a)).

### 3.3. Indirect snap-through instability

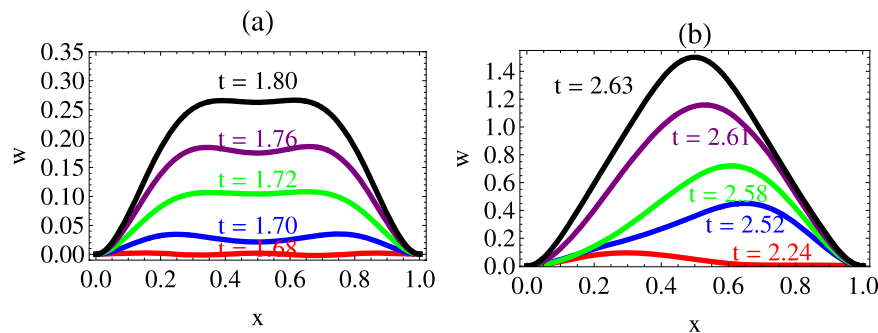
For arch height  $h = 0.5$ , figure 9 shows time histories of the mid-span deflection of the arch for different values of the step potential difference  $\beta$ . At  $\beta = 293$  the arch undergoes several cycles of moderate-amplitude oscillations before snapping. The pull-in instability occurs immediately after the snap-through instability. The critical value of  $\beta$  is between 292 and 293. This behavior is significantly different from that observed for  $h = 0.3$  and  $0.35$  for which the snap-through instability ensued without the arch undergoing oscillations.

Figure 10 exhibits the deformed arch at different non-dimensional times for  $\beta = 292$  (before the snap-through) and 293 (after the snap-through). It is clear that the arch

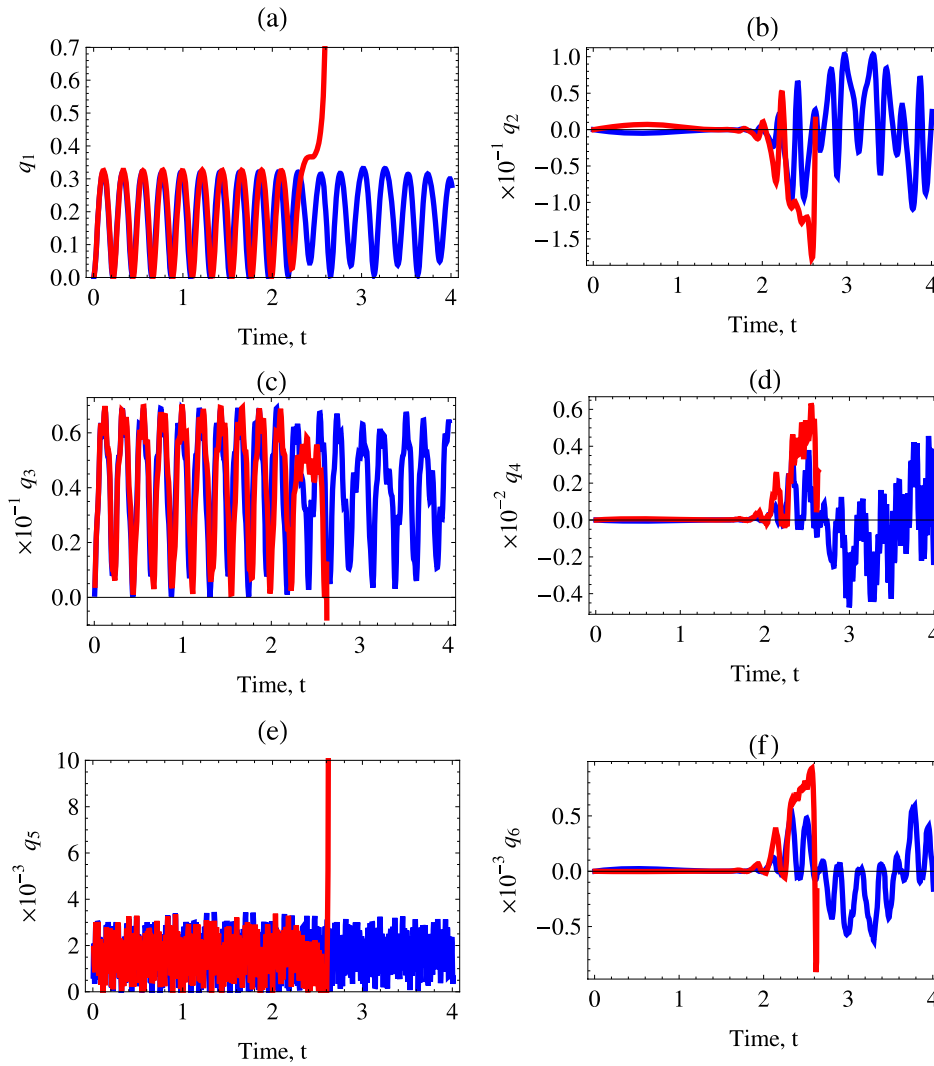
deforms symmetrically about  $x = 0.5$  before the snap-through instability but asymmetrically after the snap-through. However, we observe that just before the arch touches the bottom electrode at  $t = 2.63$ , its deflections are again symmetric about  $x = 0.5$ . Figure 11 exhibits time histories of the generalized coordinates for  $\beta = 292$  and 293. These show that the participation of the asymmetric modes ( $q_2, q_4$  and  $q_6$ ) grows considerably after  $t = 2.2$  when the snap-through instability occurs. Whereas values of  $q_2, q_4$  and  $q_6$  for the snap-through instability discussed in section 3.2 were of the order of  $10^{-11}$ , those in figure 11 are of the order of  $10^{-1}$ ,  $10^{-2}$  and  $10^{-3}$ , respectively. Asymmetric modes do start to participate in deformation of the arch for  $\beta = 292$  after  $t = 2.2$ . However, participations of the asymmetric modes remain bounded and do not cause either the snap-through or the pull-in instability. On the other hand, for  $\beta = 293$ , the displacement of the arch becomes unbounded at about  $t = 2.2$  and the pull-in instability occurs. These suggest that the participation of mode 2 is more than that of modes 4 and 6 in making deformations of the arch asymmetric about  $x = 0.5$ .

The reason for the snap-through instability with asymmetric deformations is the parametric excitation of the antisymmetric modes arising through coupling terms such as  $q_i q_{i+1}$  (with  $i = 1, \dots, n-1$ ) in equation (6). These terms act as an effective load whose magnitude increases as the amplitude of the antisymmetric motion grows under the parametric excitation. If the amplitude of antisymmetric motion becomes sufficiently large, a critical effective load is reached and the snap-through instability occurs. This second kind of snapping is called ‘indirect snap-through’ or ‘parametrically induced snap-through’ in [27] where snapping of a shallow arch under a displacement-independent step pressure load is studied. Note that for the electric loading  $F_i$  in equation (6) also depends on deformations of the arch, unlike for the displacement-independent pressure load where  $F_i$  is independent of  $q_i$ ; thus, there are more coupled terms in the MEMS problem than those in the mechanical problem analyzed in [27]. Results for a problem reported in the next section show that, under an electric load, an arch is more prone to snap-through asymmetrically than under displacement-independent mechanical loads.

Figure 12 depicts loci of the maximum deflection produced by a step electric potential for the transient problem, and bifurcation curves for static problems obtained with the



**Figure 10.** Snapshots of the displacement of the arch ( $h = 0.5$ ) for (a)  $\beta = 292$  and (b)  $\beta = 293$ .



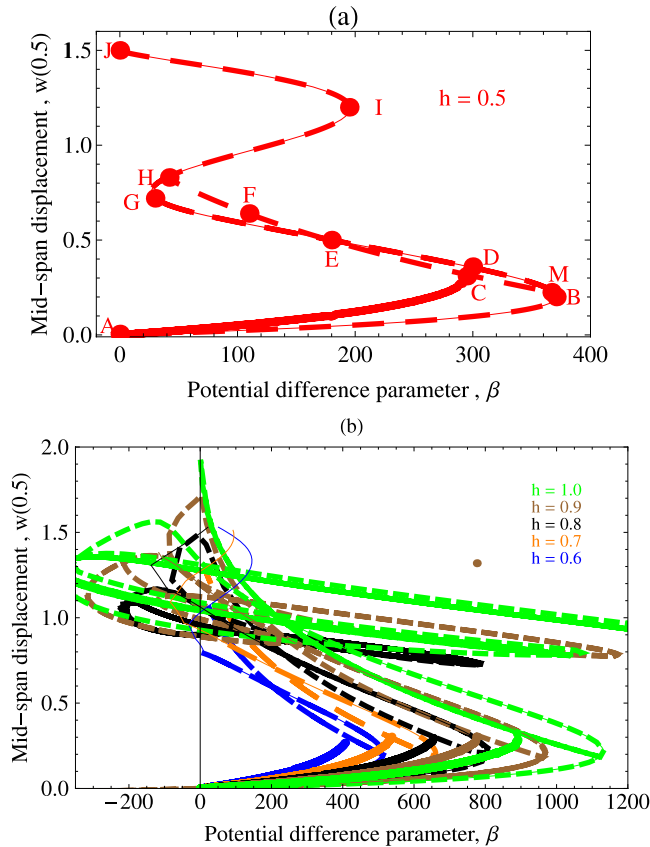
**Figure 11.** Time histories of  $q_1, q_2, \dots, q_6$  for  $\beta = 292$  (blue curves) and  $\beta = 293$  (red curves).

DIPIE and the PALC algorithms for  $h = 0.5, 0.6, 0.7, 0.8, 0.9$  and  $1.0$ . In every case, the bifurcation curve for symmetric deformations computed with the DIPIE algorithm overlaps that obtained with the PALC algorithm (e.g. the curve ABMDEGHIJ for  $h = 0.5$  in figure 12(a)). The DIPIE algorithm does not give asymmetric solutions [5, 33] (e.g. the dashed curve MCEFH for  $h = 0.5$  in figure 12(a)), which are obtained by the PALC algorithm. We note that, for all values of  $h$ , the snap-through due to the step load occurs when the locus of the maximum deflection intersects the unstable branch of the static bifurcation curve (e.g. point C for  $h = 0.5$  in figure 12(a)) corresponding to the asymmetric solution. The pull-in instability happens immediately after the snap-through instability. Values of  $\beta$  and the peak deflection  $w$  at the snap-through instability due to a step potential difference (e.g. point C for  $h = 0.5$ ) for the dynamic problem are  $\sim 20\%$  lower and  $\sim 25\%$  higher, respectively, than those for the corresponding static problem (e.g. point B for  $h = 0.5$ ). For  $h \geq 0.8$ , complex branches of the bifurcation diagram corresponding to unstable deformations of the arch are found from the PALC algorithm. A similar behavior, known as the looping behavior

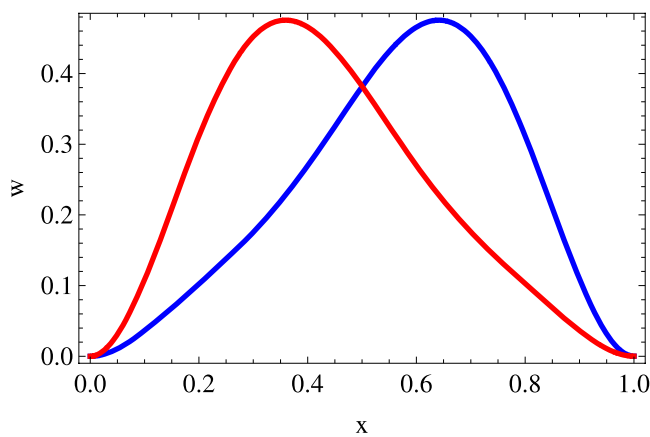
of an arch, is reported in [15, 45] due to displacement-independent mechanical pressure load. Results in figure 12(b) for  $\beta < 0$  are not valid because  $\beta$  cannot be negative. For asymmetric deformations of the arch corresponding to a point on the dashed curve BCEFH for  $h = 0.5$  in figure 12(a) the PALC algorithm gives two solutions displayed in figure 13.

In [21], the symmetry breaking of arches under electric loads in static problems is studied using a force control method and taking  $n = 15$  in equation (3). Figure 14 compares the presently computed bifurcation curve for an arch with  $h = 0.5$ ,  $\alpha = 250$  and  $n = 6$  with that reported in [21]. The bifurcation curve (ABEC from our code and AFGEC from [21]) computed with the DIPIE algorithm fails to show asymmetric deformations of the arch. Asymmetric deformations (BDC) and the looping behavior are observed in the solution from the PALC algorithm. In [21], the symmetry breaking is found with the force control method. The critical value of  $\beta$  at the snap-through instability computed with our code is  $16.7\%$  less than that reported in [21]. We found that increasing  $n$  from 6 to 10 does not change results appreciably and including mode shapes for  $n > 10$  increases numerical





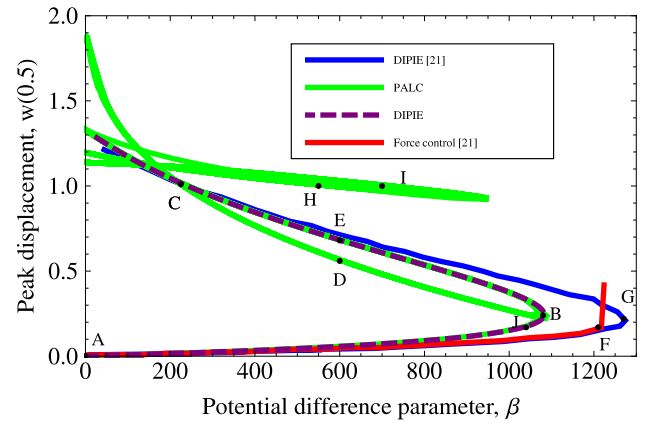
**Figure 12.** Bifurcation diagrams ( $w(0.5)$  versus  $\beta$ ) of the arch for different arch heights for static problems. Dashed curves and light solid curve are results from the DIPIE and the PALC algorithms, respectively. Dark solid curves are the loci of the maximum displacements  $\text{Max}_{0 \leq t \leq 4} (w(0.5, t))$  under the step load. Results of the DIPIE and the PALC algorithms overlap only when the arch's deformations are symmetric about  $x = 0.5$ .



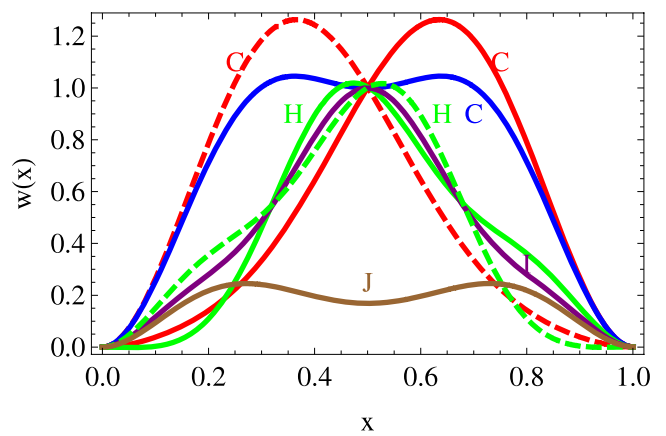
**Figure 13.** The two asymmetric solutions for  $h = 0.5$  and  $\beta = 250$ .

inaccuracies because for large  $\lambda_i \cos(\lambda_i x)$  is very small as compared to values of  $\sinh(\lambda_i x)$  and  $\cosh(\lambda_i x)$ . Present computations also show looping behavior of the arch; however, these curves (e.g. marked H and I) correspond to unstable equilibrium shapes of the arch.

Figure 15 exhibits several unstable and one stable solution for the arch corresponding to different points on the bifurcation



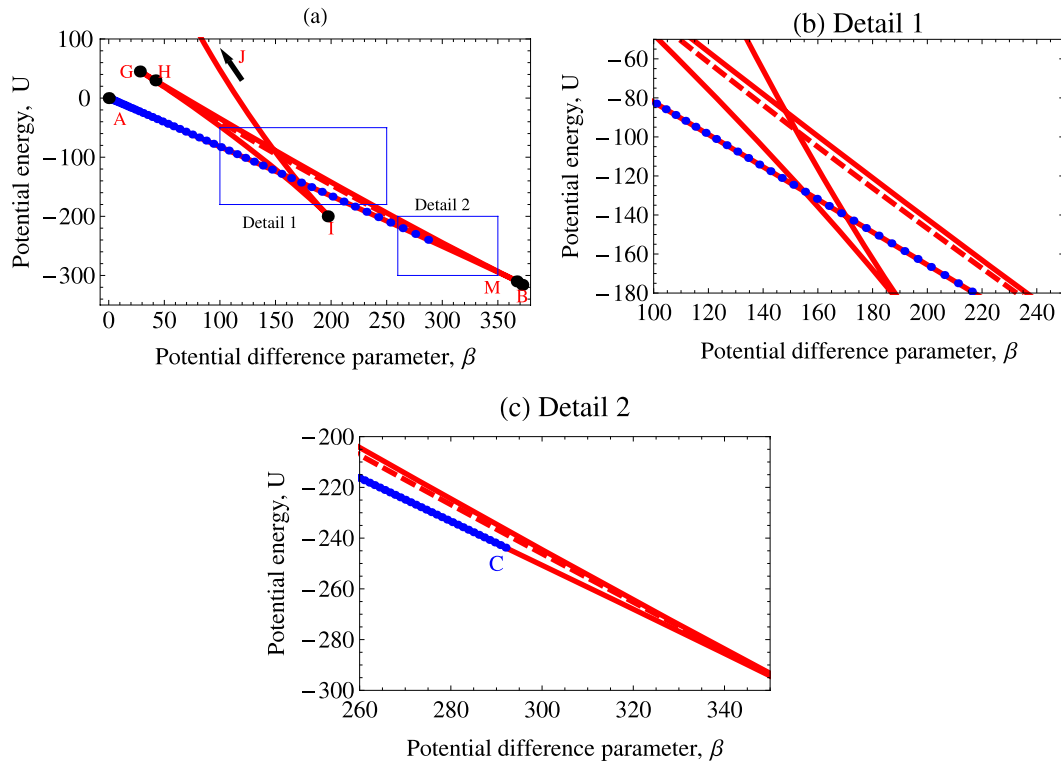
**Figure 14.** Bifurcation diagrams ( $w(0.5)$  versus  $\beta$ ) of the arch for different arch heights for static problems ( $h = 0.5$ ,  $\alpha = 250$ ).



**Figure 15.** Deformations at different points of the bifurcation diagram for  $h = 1.0$  and  $\alpha = 110$ .

curve in figure 14. Deformations corresponding to points C, H and I are unstable, which means the arch may have those equilibrium configurations under a static load of  $\beta = \sim 230$ ,  $\sim 550$  and  $\sim 700$ , respectively. However, a small perturbation will cause the arch to experience the pull-in instability. Points C and H represent multiple symmetric and asymmetric deformations of the arch.

Figure 16 exhibits the variation of  $U$  versus  $\beta$  for a static problem with  $h = 0.5$  and  $\alpha = 110$ . In figure 16, we have also plotted for transient problems the locus of the minimum values of  $U$  for step voltages  $\beta$ . Points A, B, C, etc. in figure 16 have the same values of  $\beta$  as those in figure 12(a). For a static problem,  $U$  decreases from 0 at point A to  $\sim -300$  at point B as  $\beta$  increases from 0 to 370. We note that there is no other solution with a lower value of  $U$  than that at point B. At point B, the arch experiences the snap-through instability followed immediately by the pull-in instability and no solution exists for a value of  $\beta$  that is higher than the value of  $\beta$  at point B. We also note that, for  $h = 0.35$  (see figure 11), the value of  $\beta$  at point I is higher than the value of  $\beta$  at point B, whereas for  $h = 0.5$  (see figure 16), the value of  $\beta$  at point I is lower than that at point B. In figure 16(b), the total potential energy for an unstable asymmetric solution corresponding to a point on the

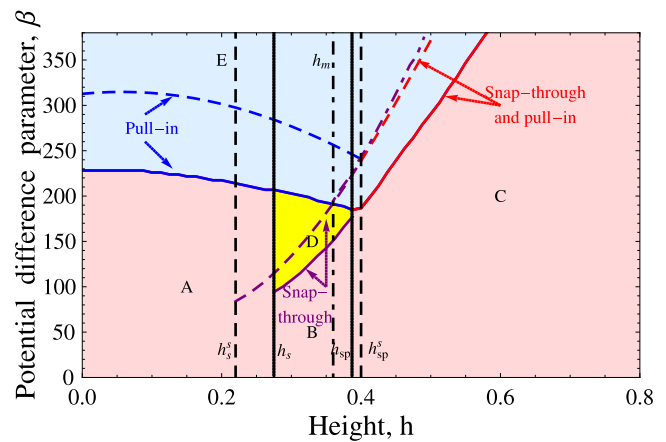


**Figure 16.** Potential energy  $U(\beta)$  versus the potential difference parameter  $\beta$  for  $h = 0.5$  for the static problem (red curve) and the locus of the minimum potential energy  $\text{Min}_{0 \leq t \leq 2}(U(\beta, t))$  as a function of the step potential difference for the dynamic problem (blue dots).

dashed curve MH is lower than that for the unstable symmetric solutions represented by the solid curve BH. The locus of the minimum potential energy for the dynamic problem denoted by blue dots in figure 16 follows the curve of  $U$  versus  $\beta$  for the static problem from point A to point C. At point C, the snap-through instability and the pull-in instability happen simultaneously for the dynamic problem.

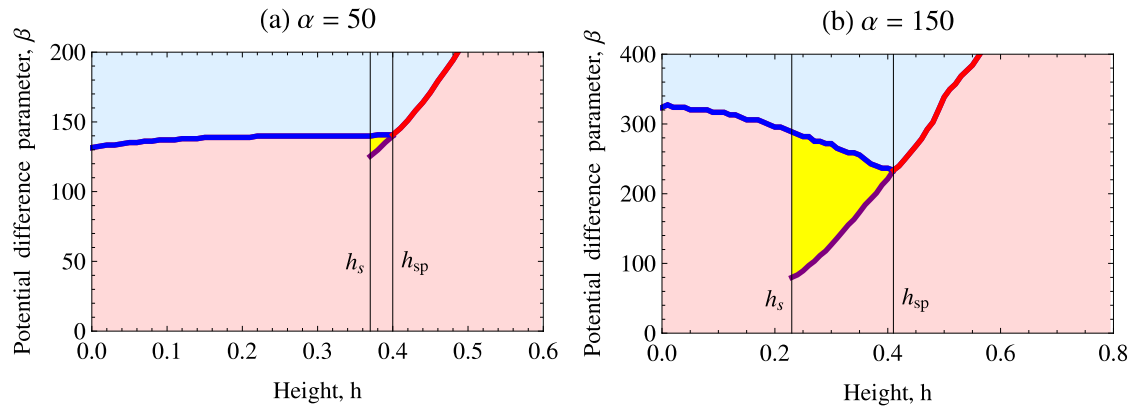
### 3.4. Comparison of results for dynamic problems with those for static problems

The critical potential difference parameters for the pull-in instability (blue curve), the snap-through instability (purple curve) and the snap-through followed immediately by the pull-in instability (red curve) are plotted as a function of the arch height in figure 17 for  $\alpha = 106.0$ . Solid and dashed curves correspond, respectively, to results for dynamic and static problems due to the application of the step electric potential difference. Note that only values of  $h$  and  $\beta$  are varied and other parameters for the MEMS are kept unchanged. For  $h$  between 0.0 and 0.8, the critical values of  $\beta$  for dynamic problems are ‘substatic’ in the sense that they are lower than those for the corresponding static problems. The minimum height  $h_s^s = 0.22$  required for the arch to experience the snap-through instability for a static problem is less than the corresponding value of  $h_s = 0.275$  for the dynamic problem. However, the minimum arch heights at which the blue and the purple curves meet each other are almost the same for the static and the dynamic problems (e.g.  $h_{sp}^s = 0.4$ ,  $h_{sp} = 0.37$ ). An arch with a height greater than  $h_{sp}$  will experience the pull-in



**Figure 17.** Critical values of  $\beta$  for the snap-through (purple curve) and the pull-in instabilities (blue curve) to occur for an arch with  $\alpha = 106.0$ . The red curve shows critical values of  $\beta$  for which the snap-through and the pull-in instabilities occur simultaneously. Solid and dashed curves correspond, respectively, to results for the dynamic and the static problems. The dotted–dashed purple curve represents results for the dynamic problem with a displacement-independent load.

instability immediately after the snap-through instability under a step electric potential difference;  $h_{sp}^s$  is the corresponding value for the static problem. The dotted–dashed purple curve shows critical values of  $\beta$  for the snap-through instability due to a step displacement-independent mechanical load given by equation (15).



**Figure 18.** Critical values of  $\beta$  for the snap-through (purple curve) and the pull-in instabilities (blue curve) occur for an arch with (a)  $\alpha = 50.0$  and (b)  $\alpha = 150.0$ . The red curve shows the critical values of  $\beta$  for which the snap-through and the pull-in instabilities occur simultaneously.

The minimum arch height  $h_m$  required for the snap-through instability to occur for this mechanical load is greater than  $h_s$  for the displacement-dependent step electric load. Therefore, arches under electric loads are more prone to the snap-through instability and results from studies of the response of an arch for a displacement-independent load cannot be directly used to predict the response of the same arch under an electric load.

The curves of critical values of  $\beta$  for step electric loads divide the plot of  $\beta$  versus  $h$  in figure 17 into three regions. An arch of height  $h$  under a potential difference  $\beta$  from regions A, B and C (light red) will experience oscillations of relatively small amplitude and time period without experiencing a snap-through instability. On the other hand, an arch of height  $h$  under a potential difference  $\beta$  from region D (yellow) will experience oscillations of large amplitude and large time period after undergoing the snap-through. However, an arch under a potential difference  $\beta$  from region E (light blue) will not have any stable motion; it will experience the pull-in instability and will touch the bottom electrode.

Figure 18 exhibits plots of  $\beta$  versus  $h$  for dynamic problems and two values, 50 and 150, of  $\alpha$ . Results in figure 18 for the two values of  $\alpha$  are quantitatively different but are qualitatively similar to those for  $\alpha = 106$  in figure 17. For different values of  $\alpha$ , table 1 lists values of the minimum heights  $h_s$  and  $h_{sp}$  required for the snap-through and the combined snap-through and pull-in instabilities, respectively. The value of  $h_s$  decreases with an increase in the value of  $\alpha$ ; however, that of  $h_{sp}$  remains the same. The size of the yellow region increases with an increase in the value of  $\alpha$ . For small values of  $\alpha$ , this region will vanish. Figures 17 and 18 provide a qualitative idea of the phase diagram of a micro-arch under a step electric load and give quantitative information for  $\alpha = 50.0, 110.0$  and  $150.0$ . Similarly, one can construct phase diagrams for other values of the stretching parameter  $\alpha$ .

### 3.5. Comparison of present results with those from a continuum-mechanics approach

In [12] transient finite plane strain elastodynamic deformations of a perfect electrically conducting undamped clamped-clamped bell-shaped arch, of the same dimensions and

**Table 1.** For different values of  $\alpha$ , minimum values of the arch height for the snap-through and the combined snap-through and pull-in instabilities occur.

	$\alpha$		
	50	110	150
$h_s$	0.37	0.28	0.23
$h_{sp}$	0.4	0.4	0.4

material as the arch in section 3.1, suspended over a flat rigid semi-infinite perfect conductor, are analyzed with a continuum-mechanics approach. The coupled nonlinear partial differential equations for mechanical deformations are solved numerically by the finite element method and those for the electrical problem by the boundary element method. Effects of geometric nonlinearities are incorporated in the problem formulation and solution; however, structural damping and the damping due to the interaction of the structure with the surrounding medium are neglected. Table 2 compares present results with those reported in [12] for a step potential difference between the two electrodes. It is clear that the present reduced-order model with six degrees of freedom gives results very close to those reported in [12] with thousands of degrees of freedom (recall that the problem in [12] was solved numerically with numerous nodes). For a complex shaped MEMS for which mode shapes are not readily available, one needs to either find mode shapes numerically or use the approach of [12]. The snap-through instability reported in [12] is the direct snap-through instability.

In an attempt to compare asymmetric solutions from the two approaches, we study deformations of a bell-shaped silicon arch with base length  $\hat{L} = 1$  mm, width  $\hat{b} = 30$   $\mu\text{m}$ , thickness  $\hat{d} = 3.0$   $\mu\text{m}$ , initial gap  $\hat{g}_0 = 10.0$   $\mu\text{m}$ , arch rise  $\hat{h} = 10.0$   $\mu\text{m}$  and its bottom surface described by  $\hat{z}_0 = \hat{h} \sin^2(\pi \hat{x} / \hat{L})$  using the continuum-mechanics approach described in [12]. The arch experiences the indirect snap-through under a step electric potential difference. The critical value of the potential difference for the snap-through instability is between 244 and 246 V from the continuum-mechanics analysis and between 258.0 and 258.2 V from the present model. The difference of 5% between the two values attests

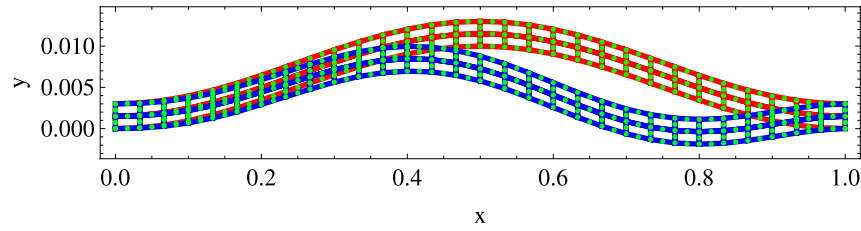


Figure 19. Snapshots of the arch at  $t = 0$  (red) and  $t = 250$  s (blue).

Table 2. Comparison of present results with those from the continuum-mechanics approach.

	Pull-in voltage	Pull-in displacement	Snap-through voltage	Peak displacement before the snap-through
Present model	Between 95 and 96 V	11.2 $\mu\text{m}$	Between 65 and 66 V	2.66 $\mu\text{m}$
Continuum-mechanics-based analysis [12]	Between 92 and 93 V	11.0 $\mu\text{m}$	Between 65 and 66 V	2.60 $\mu\text{m}$
Difference	3.2%	1.8%	0%	2.3%

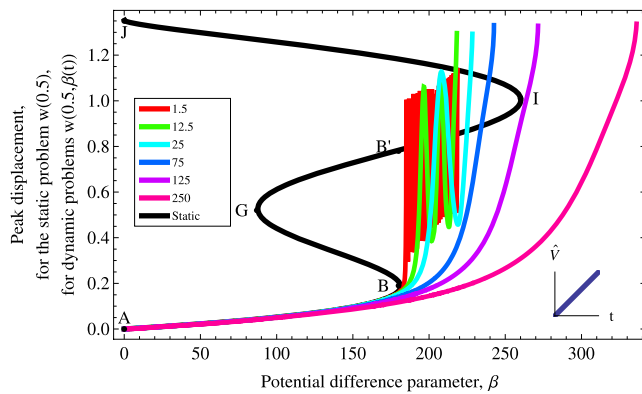


Figure 20. For different rates of increase of the applied potential difference (V/unit time) the peak displacement of the mid-span of the arch versus the potential difference parameter  $\beta$ .

to the adequacy of the present reduced-order model. Figure 19 exhibits the arch configurations at two different times obtained from the continuum mechanics formulation of the problem. It is clear that the arch deformed asymmetrically subsequent to the snap-through. Thus, the continuum mechanics approach also predicts the indirect snap-through with deformations of the arch asymmetric about  $x = 0.5$ .

### 3.6. Snap-through and pull-in instabilities under a potential difference linearly varying with time

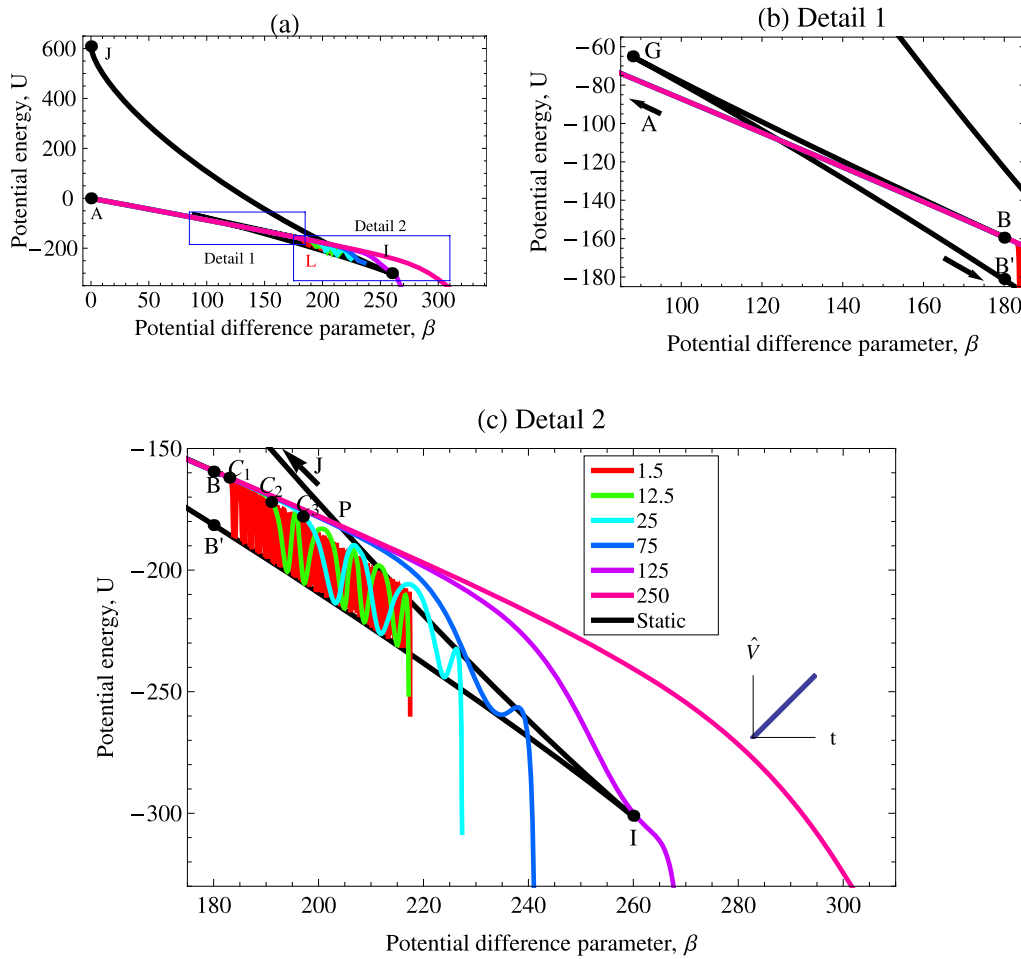
The static bifurcation curve ABGB'IJ in figure 20 for an arch with  $h = 0.35$  and  $\alpha = 110$  obtained from the PALC algorithm shows that the arch undergoes both the snap-through instability near point B and the pull-in instability near point I. For low to medium rates (i.e.  $\leq 25$  V/(unit time)) of increase of  $\beta$ , the arch vibrates around the snapped-through shape until the pull-in occurs. For a bell shaped arch with  $\hat{L} = 1$  mm, width  $\hat{b} = 30$   $\mu\text{m}$ ,  $\hat{d} = 3.0$   $\mu\text{m}$ ,  $\hat{g}_0 = 12.85$   $\mu\text{m}$ ,  $\hat{h} = 4.5$   $\mu\text{m}$ ,  $\hat{E} = 169$  GPa,  $\hat{\rho} = 2231$  kg m $^{-3}$ , the

nondimensional parameters are  $\alpha = 110$ ,  $h = 0.35$  and  $t = \hat{t}/\sqrt{(\hat{E}\hat{I})/(\hat{\rho}\hat{b}\hat{d}\hat{L}^4)} = \hat{t}/7537.45$  where  $\hat{t}$  is in second. For loading rates  $> 75$  V/(unit time), the snap-through instability is not observed. As the rate of increase of  $\beta$  is decreased the snap-through voltage gradually decreases to 183 V and the pull-in voltage approaches 215 V. It is evident that the snap-through parameters of the arch approach those of the statically deformed arch as the loading rate is decreased. However, the static pull-in voltage exceeds by  $\sim 20\%$  the pull-in voltage for the slowest loading rate considered here.

Figure 21 exhibits the variation of  $U$  (cf equation (15)) versus  $\beta$  for dynamic and static problems.  $U$  for dynamic problems follows the curve for the static problem from point A to point B for every rate of increase of  $\beta$ . After point B, the snap-through instability occurs at points  $C_1$ ,  $C_2$  and  $C_3$  for loading rates of 1.5, 12 and 15 (V/unit time), respectively. After the snap-through, the arch vibrates around the new equilibrium position and the potential energy for the dynamic problem always remains higher than that for the static problem (curve B'I in figure 21(c)). For loading rates  $\geq 75$  V/unit time, the curve  $U$  versus  $\beta$  intersects the curve IJ of  $U$  versus  $\beta$  for the static problem at point P. For loading rates  $\geq 75$  (V/unit time), no snap-through instability is observed. For the loading rate of 75 (V/unit time), the arch experiences the pull-in instability just after its potential energy overshoots the curve IJ. Therefore, the stable motion of the arch under a linearly varying electric potential is observed only when  $U$  for the dynamic problem  $\geq U$  of stable deformations (B'I) for the static problem and  $\leq U$  of unstable (IJ) deformations of the static problem.

### 3.7. Discussion

Simitses [40] has discussed three methods for estimating critical conditions for elastic structures due to transient loads. In the first method governing equations of the problem are solved and a critical condition is reached if a small change in



**Figure 21.** Total potential energy versus the potential difference parameter  $\beta$  for  $h = 0.35$  for the static problem (black curve) and the variation of the total potential energy as a function of the potential difference parameter for the dynamic problem when the electric potential is increased linearly with time (colored curves).

a load parameter causes a large change in the response of the structure. Here, we have followed this approach and solved governing equation (1) using various numerical approaches and estimated the critical values of  $\beta$  for the snap-through and the pull-in instabilities by monitoring any large change in the peak displacement of the arch for a small change in  $\beta$ .

In the second method, the critical condition of a structure is established by studying the total energy-phase plane for the problem [22, 23], and in the third method the critical load is estimated by analyzing variations of the potential energy due to a small change in the load parameter [45]. The variations of the potential energy of the arch with  $\beta$  for the step electric potential difference reported in the previous sections uses the third method for instability analysis.

In [40], the snap-through instability of shallow sinusoidal arches under transient displacement-independent load is studied using  $n = 2$  in equation (3). The pressure over the span of the arch is assumed to vary sinusoidally, and the governing equation is expressed in terms of two non-dimensional parameters, namely the arch stiffness and the load. The lower and the upper bounds of the critical value of the load parameter for the snap-through instability are studied for the step and impulsive loads.

Deformations of the arch MEMS are governed by four non-dimensional parameters. Instead of quantifying critical values of the load parameter, we have focused on studying mechanisms of instabilities of arches under displacement-dependent electric load.

#### 4. Conclusions

We have investigated the snap-through and the pull-in instabilities in an electrically actuated micro-arch modeled as an undamped Euler–Bernoulli beam incorporating the nonlinear mid-plane stretching. Two distinct mechanisms, namely the ‘direct’ and the ‘indirect’, snap-through instabilities are found. Whereas the PALC algorithm can compute multiple branches in the bifurcation curve, which correspond to symmetric and asymmetric deformations of the arch, the DIPIE algorithm fails to compute asymmetric solutions.

The phase diagram between the critical load parameter and the arch height showing stable and unstable regions of the arch’s deformations will help in designing arch-shaped MEMS.

Results, including the indirect snap-through and asymmetric deformations subsequent to the snap-through instabil-



ity from the present reduced-order model agree well with those from the continuum-mechanics-based approach.

Other conclusions are summarized below.

- (1) An undamped arch under a step electric load may experience either direct or indirect snap-through instability.
- (2) For relatively small arch heights (e.g. the non-dimensional  $h \leq 0.25$  for  $\alpha = 106$ ), the static problem has solutions with deformations symmetric about the mid-span of the arch and the direct snap-through happens when the locus of the maximum deflection of the dynamic problem intersects the unstable branch of the bifurcation curve for the static problem.
- (3) For relatively large arch heights (e.g.  $h > 0.25$  for  $\alpha = 106$ ), the static problem has solutions with deformations symmetric and asymmetric about the mid-span of the arch. The asymmetric solution has lower total potential energy than the corresponding symmetric solution.
- (4) The indirect snap-through happens when the locus of the maximum deflection of the dynamic problem intersects the unstable branch of the bifurcation curve of the asymmetric deformations for the static problem.
- (5) For a dynamic problem with the electric potential difference applied as a step function or as a linear function of time, the arch has a stable motion only when its minimum potential energy is  $\geq$  the potential energy of the stable deformed shape of the static problem and  $<$  is the potential energy of the unstable deformed shape of the static problem.

## Acknowledgments

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